RICHMOND CITY NORTH PAVILION

LOCATION: NORTH WESTERLY CORNER OF PARCEL NUMBER 09-058-0016

Site plan
Scale: 1" = 20'

N

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SD1 STRUCTURAL DETAILS
SD2 STRUCTURAL DETAILS
SD3 STRUCTURAL SCHEDULE & NOTES

DO NOT SCALE 11"X17" PRINTS
THEY ARE NOT TO SCALE

OWNER: JAY L. BAIR
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EMAIL: jay@bairdesigns.com
WEB: bairdesigns.com
336 SUNBURST LN.
RICHMOND, UT, 84333

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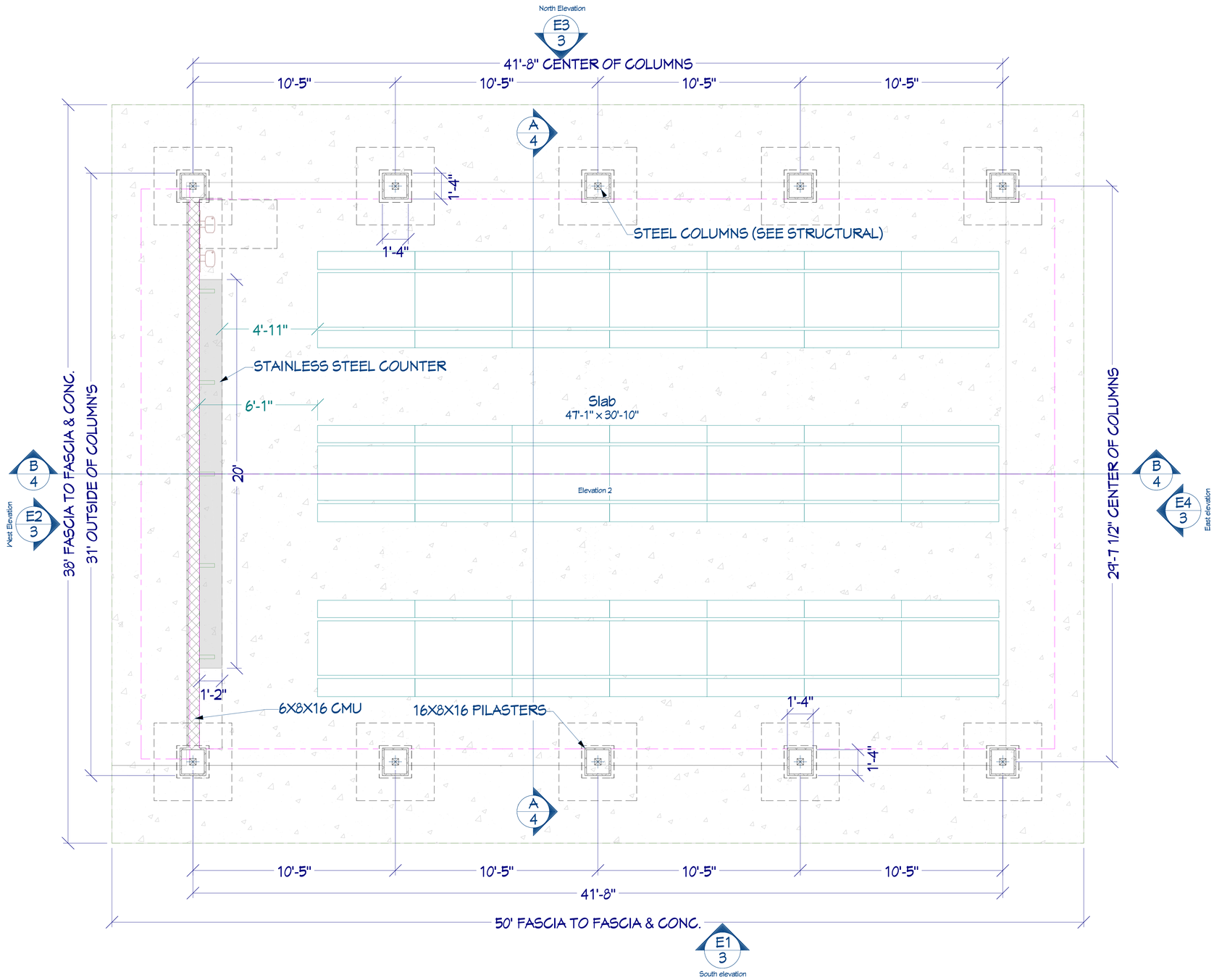
PROJECT FOR:
Richmond City

TITLE:
Site plan

DATE:
3/28/2023

PAGE #
1



Pavilion floor plan
Scale: 3/8" = 1'

This configuration can seat 72/96 adults.

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Pavilion floor plan

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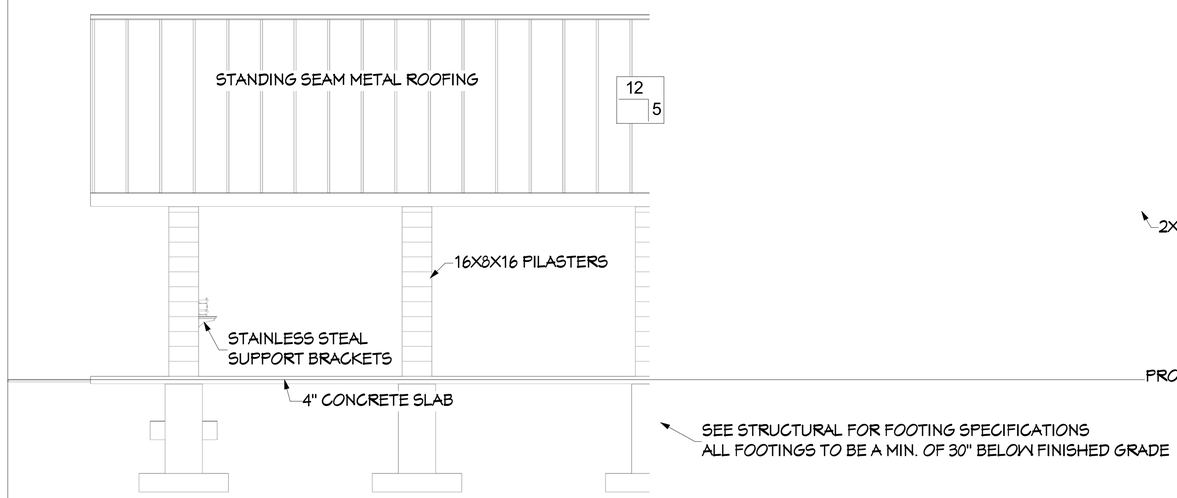
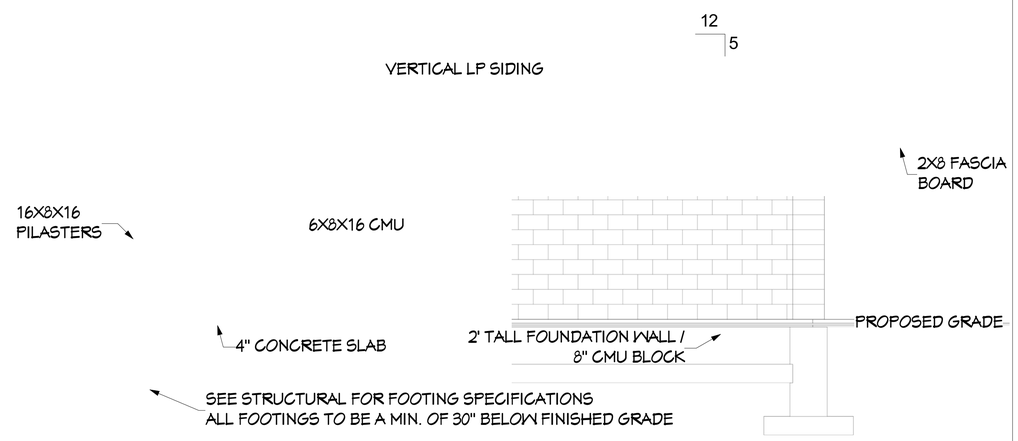
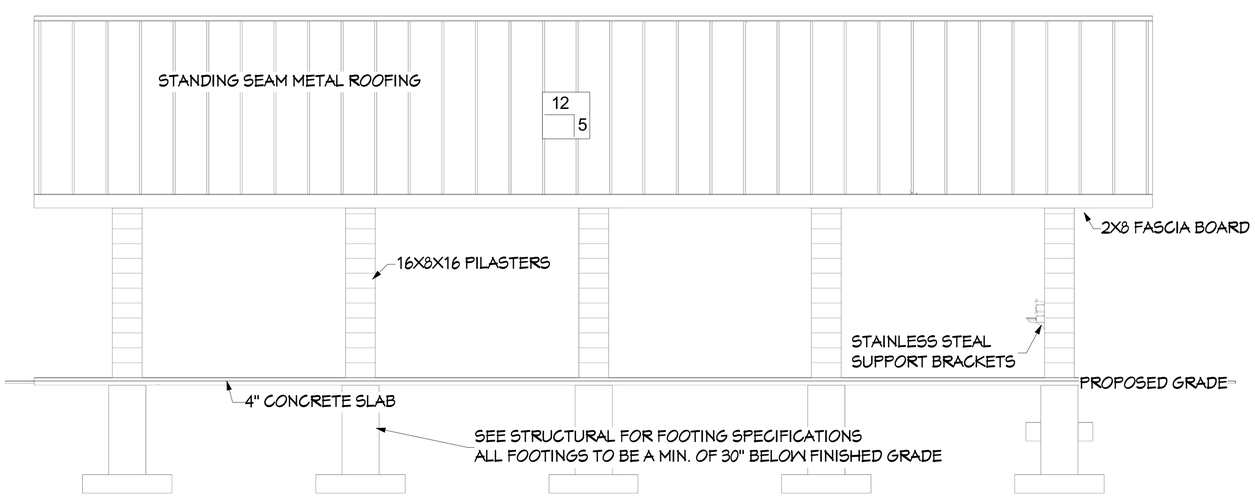


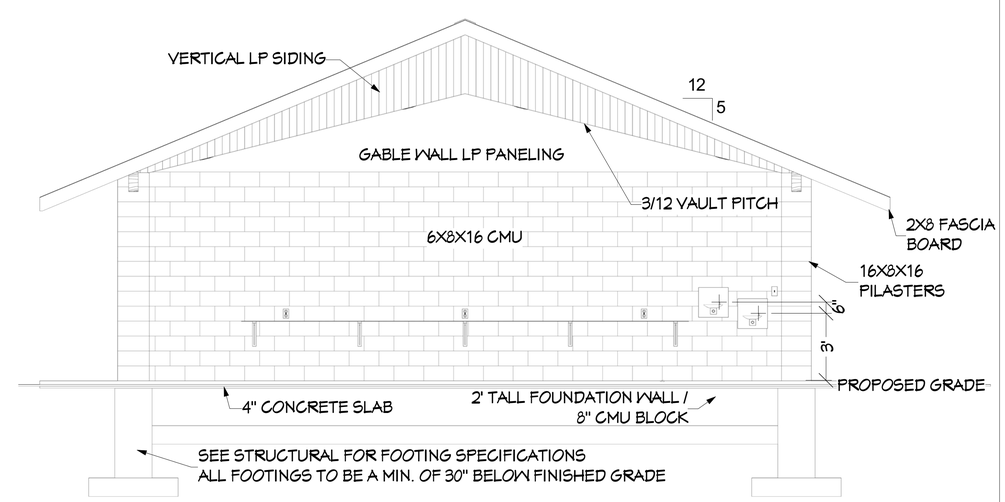
E1 South elevation
Scale: 5/16" = 1'



E2 West elevation
Scale: 5/16" = 1'



E3 North elevation
Scale: 5/16" = 1'



E4 East elevation
Scale: 5/16" = 1'

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TITLE: Elevations

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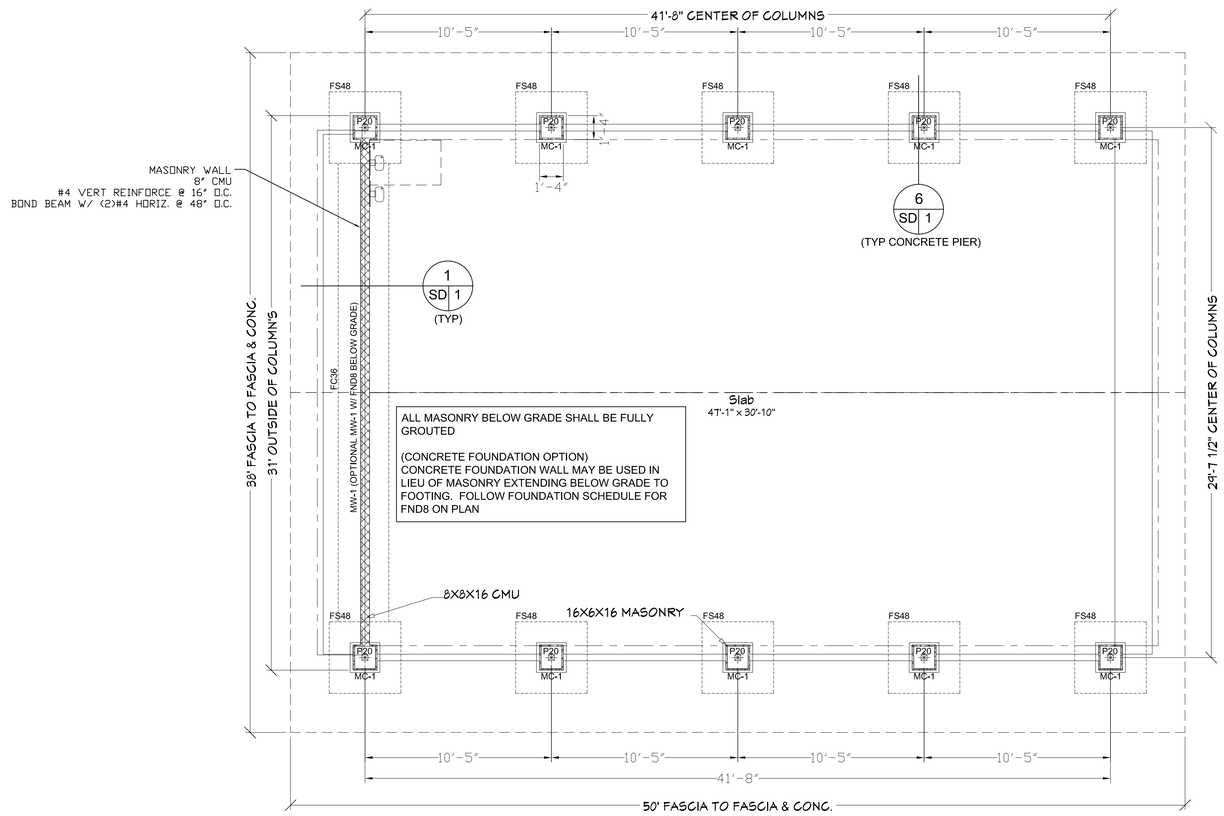


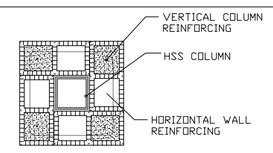
MASONRY WALL SCHEDULE

WALL	THICKNESS	MATERIAL	SOLID GROUT	REINFORCING VERTICAL	REINFORCING HORIZONTAL	JOINTS	COMMENTS
MW-1	8"	CMU	NO	#4 AT 16" O.C.	(2)#4 AT 48" O.C.	NO	PROVIDE (2) #4 HORIZ TOP

MASONRY WALL NOTES:
1. COORDINATE WALL FINISHES, MATERIALS, COURSING, ETC. WITH ARCHITECTURAL DRAWINGS.
2. DO NOT SOLID GROUT WALLS UNLESS REQUIRED BY SCHEDULE, NOTES, OR DETAILS.
3. SOLID GROUT ALL MASONRY COURSES BELOW GRADE.
4. VERTICAL REINFORCING SHALL BE CENTER IN WALL (UND).
5. PROVIDE TWO VERTICAL BARS (MIN) AT ALL CORNERS AND END OF WALLS.
6. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS.
7. 8" MASONRY IS ASTM C90 MEDIUM WIEGHT, GRADE N-1
8. PROVIDE TYPE 'S' MORTAR PER IBC TABLE 2103.7
9. PROVIDE GROUT WITH A COMPRESSIBLE STRENGTH OF 1500 PSI @ 28 DAYS
10. FOR SIGN WALL, PROVIDE A JOINT IN THE CMU WALL MID LENGTH

MASONRY HSS COLUMN SURROUND SCHEDULE

MARK	SIZE	REINFORCING VERTICAL	REINFORCING TIES	REINFORCING SCHEMATIC
MC-1	6"X16"	(4) #4	#4 @ 48"	SEE TYPICAL MC-1 CONFIGURATION



TYPICAL MC-1 CONFIGURATION SCHEMATIC

MASONRY COLUMN NOTES:
1. THE CENTERLINE OF THE VERTICAL BARS SHALL BE LOCATED 2 1/2" FROM THE FACE OF THE MASONRY. HORIZONTAL WALL REINFORCING SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS.
2. VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF WALL (UND).
3. VERTICAL MASONRY COLUMN REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS TALLER THAN 5'-0", VERTICAL COLUMN REINFORCING SHALL DOWEL 4'-0" MINIMUM INTO THE FOUNDATION WALL.
4. IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY COLUMN REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS THE MASONRY COLUMN ABOVE.
5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

A FOUNDATION PLAN

SCALE: 1/4" = 1'-0" FOR PLANS PRINTED 24X36
1/8" = 1'-0" FOR PLANS PRINTED 11X17

NOTE: IF ENGINEER'S STAMP AND SIGNATURE ARE NOT SHOWN IN THIS BOX, THIS PLAN SHALL BE CONSIDERED A DRAFT AND NOT FOR CONSTRUCTION

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STRUCTURAL • CIVIL
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SEE SHEET SD3 FOR CONCRETE & REINFORCEMENT SPECIFICATIONS, AND SOIL DESIGN ASSUMPTIONS

30"	MINIMUM FROST DEPTH
6"	MINIMUM FINISH GRADE TO TOP OF FOUND.

FOOTING SCHEDULE

SPEC	CONTINUOUS SECTION SIZE		REINFORCMENT	
	DEPTH	WIDTH	CONTINUOUS	TRAVERSE
FC36	10"	36"	(4) #4	#4 @ 16"
SPEC	SPOT SIZE, 10" DEPTH		REINFORCMENT	
	LENGTH	WIDTH	LONGWISE	SHORTWISE
FS48	48"	48"	(5) #4	(5) #4

NOTES
1. PROVIDE REINF. STEEL 3" FROM BOTTOM OF FOOTINGS.
2. PROVIDE CONT. BENT STEEL AROUND CORNERS AND LAP WALL STEEL 24" MIN.
3. PROVIDE A 24" MIN. LAP FOR ALL STEEL.
4. FOOTINGS NEED NOT BE CONTINUOUS THRU FOUNDATION STEPS U.NO.
5. SCHEDULE SHOWN APPLICABLE TO APPLIED PLAN ONLY.
6. FOOTINGS SUPPORTING CONCRETE FOUNDATION WALLS: PROVIDE "HOOKED" VERTICAL REINFORCEMENT EMBEDDED IN FOOTING 6" AND EXTENDING OUT OF FOOTING 24" TO LAP FOUNDATION WALL REINFORCEMENT. VERTICAL EMBEDDED REINFORCEMENT TO MATCH FOUNDATION WALL VERTICAL SPACING SPECIFICATION

PIER SCHEDULE

SPEC	CONTINUOUS SECTION SIZE		REINFORCMENT	
	DIA.	SQUARE OPTION	VERTICAL	TIES
P20	20"	20" X 20" MIN.	(4) #4	#3 @ 12" O.C.

1. EMBED VERTICAL REINFORCMENT INTO PIER FOOTING 5" MIN.
2. ALL VERTICAL REINFORCMENT WITH TIES: TIES SHALL WRAP THE OUTSIDE OF VERTICAL REINFORCEMENT ONCE AND WITH EACH END WRAPPING WITH A 270 DEGREE BEND AND TERMINATING ON THE SAME VERTICAL BAR.

FOUNDATION WALL SCHEDULE

SPEC	DESCRIPTION			
FND8	8" THICK CONTINUOUS (TYPICAL U.N.O.)			
HEIGHTS	REINFORCMENT (O.C.)		OPENING LINTELS	
	VERTICAL	HORIZONTAL	MAX SPAN	MIN DEPTH
2'-0"	#4 @ 16"	(2) #4	2'-0"	6"
3'-0"	#4 @ 16"	(3) #4	2'-0"	6"
4'-0"	#4 @ 16"	(4) #4	3'-0"	6"

1. PROVIDE REINF. STEEL WITHIN 4" OF TOP & BOTTOM OF WALL AND WITHIN 2" OF OPENINGS.
2. PROVIDE (2) #4 ABOVE OPENINGS, AND (1) #4 EACH SIDE AND BELOW.
3. PROVIDE 3" OF MIN. CONC. COVER FOR ALL REINF. STEEL.
4. PROVIDE CONT. BENT STEEL AROUND CORNERS AND LAP WALL STEEL PER SCHEDULE (#4 - 18" MIN, #5 - 20" MIN.)
5. PROVIDE A 24" MIN. LAP FOR ANY STEEL IF NOT NOTED.
6. ALL STEEL IS CENTERED WITHIN THE WALL THICKNESS U.N.O. OR ON DETAILING
7. EMBEDDED PIPES SHOULD HAVE A MAX. DIA. OF 2-1/2"
8. DO NOT BACKFILL FOUNDATION WALL UNTIL CONC. FLOOR SLAB IS IN PLACE AND CURED FOR 7 OR MORE DAYS.
9. SCHEDULE SHOWN APPLICABLE TO APPLIED PLAN ONLY.

PROJECT TITLE:
Richmond City North Pavilion
Approx. 200 W Main Street
TAX ID 09-058-0016
Richmond, Utah

MARK	DESCRIPTION	DATE
	DESIGN SET	02/20/2023

DESIGNED BY:
TRAVIS EVANS 435-720-2907
TRAVIS@PRICE-ENGINEERING.COM

CHECKED BY:
KYLE PRICE 801-791-6274
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LICENSED PROFESSIONAL ENGINEER
NO. 10285241
TRAVIS EVANS
STATE OF UTAH
03/28/2023

SHEET TITLE:
FOUNDATION PLAN

S1.0

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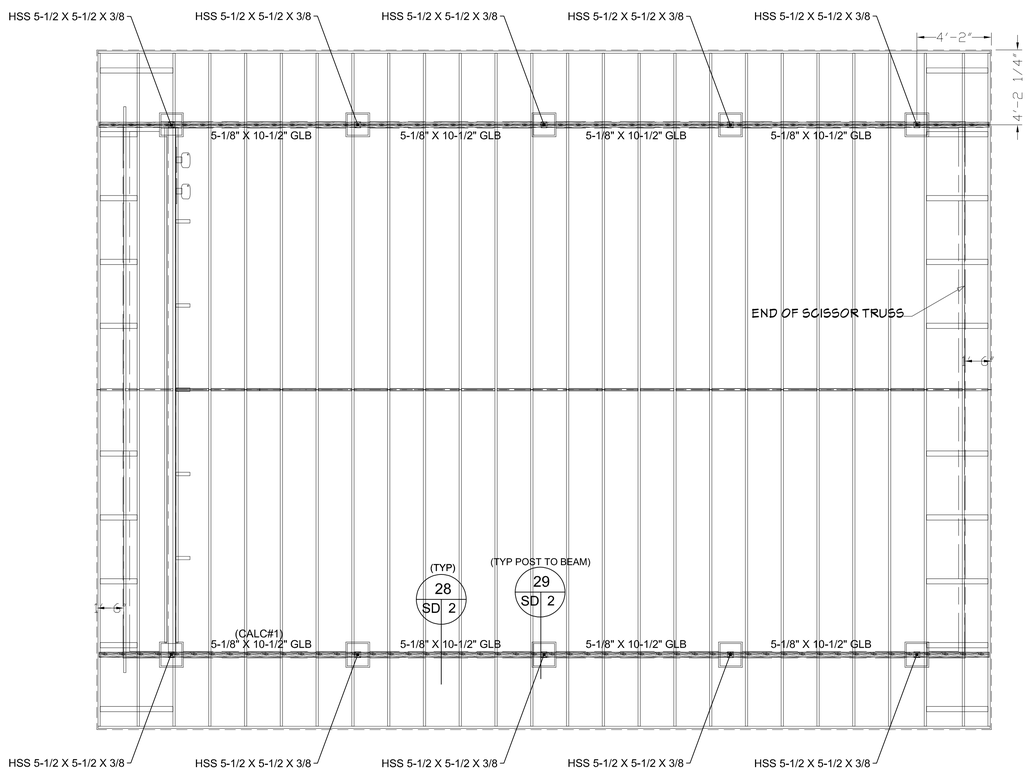
HSS 5-1/2 X 5-1/2 X 3/8 COLUMN:

5-1/2" X 5-1/2" X 3/8" HSS TUBE COLUMN (TYPICAL)

W/ 3/4"X12"X12" BASE PL
& (4) 3/4" DIA ANCHOR BOLTS.
PRECAST OR EPOXY BOLTS INTO CONC. 16" DEEP
(SEE DETAIL 6)

W/ 1/4" SADDLE CAP PL
& (2) 3/4" DIA. BOLTS (SEE DETAIL 29)



ALL TRUSS HURRICANE TIES TO BE SIMPSON #H1

B ROOF FRAMING PLAN

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BEAM SCHEDULE EX

SEE NOTES 1,2,3,4,5

DESIGNATION (SEE PLAN)	BEAM DESCRIPTION	END TRIMMER QTY. U.N.O.	TYPE U.N.O.
MIN. BEAM OR HEADER U.N.O.	(2) 2 X 10 DF#2	1	2X4
B(2)5.1/2	(2) 1-3/4" X 5-1/2" LVL	1	2X4
B(2)6	(2) 2 X 6 DF/HF#2	1	2X4
B(2)7.1/4	(2) 1-3/4" X 7-1/4" LVL	1	2X4
B(3)7.1/4	(3) 1-3/4" X 7-1/4" LVL	1	2X4
B(2)8	(2) 2 X 8 DF/HF#2	1	2X4
B(3)8	(3) 2 X 8 DF/HF#2	1	2X6
B(2)9.1/2	(2) 1-3/4" X 9-1/2" LVL	3	2X4
B(2)9.1/2	(2) 1-3/4" X 9-1/2" LVL	2	2X6
B(3)9.1/2	(3) 1-3/4" X 9-1/2" LVL	3	2X6
B(2)10	(2) 2X10" DF/HF #2	1	2X4
B(3)10	(3) 2 X 10 DF/HF#2	1	2X4
B(1)11.7/8	(1) 1-3/4" X 11-7/8" LVL	2	2X4
B(2)11.7/8	(2) 1-3/4" X 11-7/8" LVL	3	2X4
B(3)11.7/8	(3) 1-3/4" X 11-7/8" LVL	4	2X6
B(4)11.7/8	(4) 1-3/4" X 11-7/8" LVL	5	2X6
B(2)12	(2) 2 X 12 DF/HF#2	1	2X4
B(2)14	(2) 1-3/4" X 14" LVL	4	2X4
B(3)14	(3) 1-3/4" X 14" LVL	4	2X6
B(2)16	(2) 1-3/4" X 16" LVL	4	2X4
B(3)16	(3) 1-3/4" X 16" LVL	5	2X6
B(2)18	(2) 1-3/4" X 18" LVL	5	2X4
B(3)18	(3) 1-3/4" X 18" LVL	5	2X6
GLB	WOOD GLULAM BEAM.	SEE PLAN	
W#X##	STEEL WIDE FLANGE BEAM	SEE PLAN	

1. JOIN MULTIPLE PIECE BEAMS W/ (2) 16D @ 12" O.C.
2. PROVIDE METAL FRAMING CONNECTORS (I.E. SIMPSON #CB) TO TIE BEAMS GREATER THAN 6'-0" TO POSTS OR TRIMMERS.
3. LVL IS LAMINATED VENEER LUMBER, I.E., MICROLAM, VERSALAM. MIINIMUM LVL SPEC PROVIDE 1.9E AND 2600 PSI F'b OR BTR.
4. GLB IS GLULAM BEAM. PROVIDE 24F-V4 SPEC U.N.O.
5. ALL STEEL BEAMS TO MEET ASTM A992 OR Fy = 50 ksi, Fu = 65 ksi CRITERIA.
6. X-BEAM IS ROSBORO X-BEAM BRAND GLULAM BEAMS OR EQUIV.
7. ALL BEAMS AND HEADERS MAY HAVE UP TO A 1-1/2" DIAMETER HOLE THRU THE ANY LOCATION OF THE SPAN, BUT NOT WITHIN 1" OF THE TOP OF BOTTOM OF THE DEPTH.

POST & COLUMN SCHEDULE

DESIGNATION (SEE PLAN)	DRESSED SECTION SIZE (APPROX) & GRADE (1)	CAP/BASE U.N.O. (2)	BASE TO CONC. U.N.O. (2)
P4X4	3-1/2" X 3-1/2" DF#1	BC4, LCE4	ABU44Z
P4X6	3-1/2" X 5-1/2" DF#1	BC46, LCE4	ABU46Z
P6X6	5-1/2" X 5-1/2" DF#1	BC6, AC6	ABU66Z
P6X8	5-1/2" X 7-1/2" DF#1	BC_, AC6	SEE PLAN
P8X8	7-1/2" X 7-1/2" DF#1	BC_, AC6	ABU88Z
P10X10	9-1/2" X 9-1/2" DF#1	SEE PLAN	ABU1010Z
P12X12	11-1/2" X 11-1/2" DF#1	SEE PLAN	ABU1212Z
P6X6RS	6" X 6" DF#1	AC6	ABU66RZ
P8X8RS	8" X 8" DF#1	SEE PLAN	ABU88RZ
P10X10RS	10" X 10" DF#1	SEE PLAN	ABU1010RZ
P12X12RS	12" X 12" DF#1	SEE PLAN	ABU1212RZ

1. DF#1 = DOUG FIR GRADE #1 OR BETTER
2. CONNECTOR MODEL NUMBERS RELATE TO SIMPSON STRONG TIE OR USP (MITEK) BRAND WHERE APPLICABLE. OTHER CONNECTOR MODEL NUMBERS ACCEPTABLE IF ALL RATED CAPACITIES EXCEED THOSE LISTED. (LCE_) & (AC_) MODELS ARE INSTALLED IN PAIRS.
U.N.O. = UNLESS NOTED OTHERWISE (ON THE PLANS).

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LICENSED PROFESSIONAL ENGINEER
NO. 10285241
TRAVIS LEE EVANS
STATE OF UTAH
03/28/2023

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SHEET TITLE:
ROOF FRAMING PLAN

S1.1

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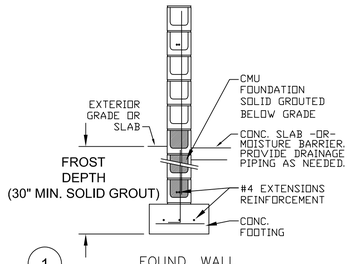

1 FOUND. WALL

NTS

2 NO DETAIL

NTS

3 NO DETAIL

NTS

4 NO DETAIL

NTS

5 NO DETAIL

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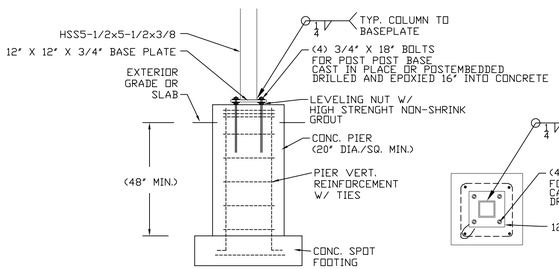

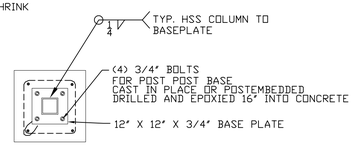

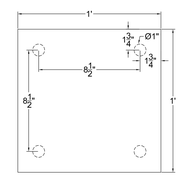

6 SPOT FOOTING & PIER PLAN VIEW STEEL BASEPLATE/BOLT TEMPLATE

NTS

7 NO DETAIL

NTS

8 NO DETAIL

NTS

9 NO DETAIL

NTS

11 NO DETAIL

NTS

12 NO DETAIL

NTS

13 NO DETAIL

NTS

14 NO DETAIL

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15 NO DETAIL

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16 NO DETAIL

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18 NO DETAIL

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19 NO DETAIL

NTS

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STRUCTURAL DETAILS

SD1

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20 NO DETAIL

NTS

21 NO DETAIL

NTS

22 NO DETAIL

NTS

23 NO DETAIL

NTS

24 NO DETAIL

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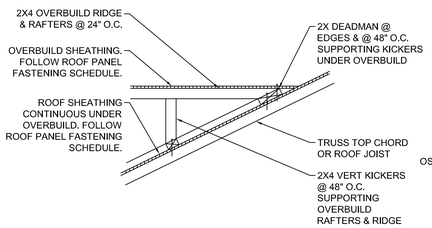
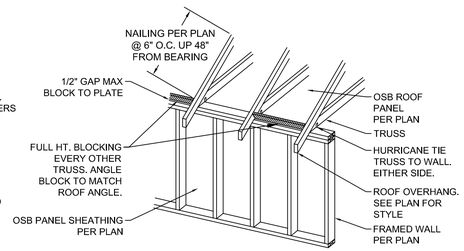


25 ROOF OVERBUILD

NTS



26 WALL TO TRUSS

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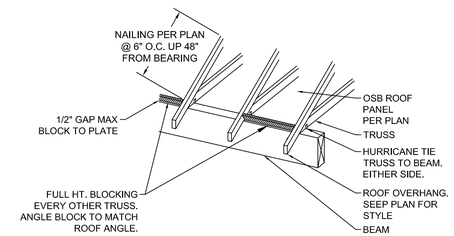

27 NO DETAIL

NTS

NO DETAIL

28 BEAM TO TRUSS

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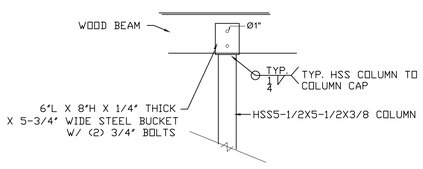

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NTS

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03/28/2023

SHEET TITLE:
STRUCTURAL DETAILS

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SOILS AND EXCAVATION NOTES

UNIFIED SOIL CLASSIFICATION: NO SOILS REPORT PROVIDED.
DESIGN SOIL BEARING PRESSURE: 1500 PSF (ASSUMED).

1. ALL DESIGN IS BASED ON STABLE SOIL CHARACTERISTICS. GEOTECHNICAL HAZARDS FOUND ON OR AROUND THE SITE OR SOILS FOUND AT THE SITE WHILE EXCAVATION OCCURS WHICH DIFFERS FROM THOSE ASSUMED SHOULD BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND ENGINEER.

2. ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR UNIFORM CRUSHED GRAVEL. SOFT AREAS DEEPER THAN 2' AND ANY ON SITE OR OTHER GEOTECHNICAL HAZARDS SHALL BE INVESTIGATED AND APPROVED BY A LICENSED GEOTECHNICAL ENGINEER.

3. DO NOT BACKFILL FOUNDATION WALLS UNTIL THE CONCRETE FLOOR SLAB IS IN PLACE AND HAS CURED A MINIMUM OF 7 DAYS.

CONCRETE SPECS & DETAILING

ELEMENT	(F'c) MINIMUM	(F'c) RECOMMENED	W/C RATIO
FOOTINGS	3000 PSI	3000 PSI	.50
WALLS	3500 PSI	3500 PSI	.50
SUSP. SLABS	3500 PSI	3500 PSI	.45
SLABS	3500 PSI	4000 PSI	.45

1. (F'c) STRENGTH BASED ON 28 DAY CURE TIME
2. PORTLAND TYPE I/II CEMENT.
3. 25% MAXIMUM CLASS "C" OR "F" FLY ASH BE WEIGHT
4. 3/4" MAX AGGREGATE SIZE
5. 4" MAXIMUM SLUMP
6. ADMIXTURES ACCEPTABLE EXCLUDING CALCIUM CHLORIDE

STEEL REINFORCEMENT DETAILING & SPECIFICATIONS	
NON-WELDABLE (ALL U.N.O.)	ASTM A 615 GRADE 60
WELDABLE	ASTM A 706 GRADE 60 LOW ALLOY
WELDED WIRE FABRIC	ASTM A 185

REINFORCEMENT CONCRETE COVER (MIN.)	INCHES
EXPOSED TO EARTH (NOT FORMED)	3"
EXPOSED TO EARTH OR WEATHER (FORMED)	2"
NOT EXPOSED TO EARTH OR WEATHER	1-1/2"

REINFORCEMENT (REBAR) LAP SCHEDULE

SIZE	MINIMUM LAP FOR CONTINUOUS REBAR, U.N.O.
#3	12"
#4	18"
#5	20"
#6	24"

1. CONTRACTOR RESPONSIBLE FOR FORMWORK AND SHORING.
2. REINFORCEMENT SUPPORT PER ACI 315
3. ROUGHEN CONSTRUCTION JOINT SURFACES TO 1/4" AMPLITUDE OR MORE
4. BEND REINFORCEMENT COLD ONCE
5. BEND RADII AND HOOK LENGTHS TO CONFORM TO ACI 318.
6. PROVIDE CONTINUOUS CORNER BARS AT WALL AND FOOTING CORNERS LONG ENOUGH TO MEET REQUIRED LAP LENGTHS.

CONCRETE PLACEMENT & CONSTRUCTION SPECS

1. VIBRATE AND CONSOLIDATE CONCRETE DURING PLACEMENT AS NEEDED TO LIMIT VOIDS AND SEPARATING OF MATERIAL INGREDIENTS.
2. WHEN PLACING CONCRETE IN HOT OR COLD WEATHER, PROVIDE PROTECTION FROM FREEZING OR ACCELERATED DRYING.
3. CONCRETE SLAB CONTROL JOINT DEPTH SHALL BE 1/3 THE SLAB THICKNESS MIN. AND SPACED NO MORE THE 30 TIMES THE SLAB THICKNESS OR PER SCHEDULE BELOW, BUT NOT MORE THAN 1.25 TIMES ANY SLAB EDGE LENGTH.

SLAB THICKNESS	JOINT DEPTH MIN	SPACING MAX
3-1/2"	1-1/4"	8'-0"
4"	1-1/2"	10'-0"
5"	1-3/4"	12'-0"
6"	2"	15'-0"

FROST DEPTH, GRADING, DRAINAGE

SEE PLAN	MINIMUM FROST DEPTH
SEE PLAN	MINIMUM FINISH GRADE TO TOP OF FOUND.

NOTES:

1. FROST DEPTH IS THE REQUIRED DEPTH OF THE UNHEATED SIDE OF THE FOUNDATION AND FOOTING, FROM ANY SURFACE OF THE ADJACENT FINISH GRADE TO THE BOTTOM OF FOOTING.
2. FINISH GRADE SHALL SLOPE DOWN AWAY FROM HOME FOUNDATIONS AT LEAST 6" OF DROP THE FIRST 10 FEET OF HORIZONTAL DISTANCE.
3. DOWNSPOUTS SHALL EXTEND OUT AWAY FROM THE FOUNDATION AT LEAST 5 FEET OR INTO AN APPROVED DRAINAGE SYSTEM OR ON SITE PIPING.
4. ALL LOT DRAINAGE SHOULD BE GRADED TO DRAIN TO APPROVED AREAS, SWALES, OR DRAINAGE SYSTEMS., PER THE REQUIREMENTS OF THE GOVERNING CITY OR COUNTY.

SHEAR WALL SCHEDULE

DESIGNATION (SEE PLAN)	OSB PANEL QTY.	8D SPACING EDGE O.C.	STUD SIZE & QTY. INSIDE EDGES ONLY. (SEE DIAGRAM BELOW)
SW6	(1) SIDE	6"	(1) 2X
SW4	(1) SIDE	4"	DBL 2X OR (1) 4X
SW3	(1) SIDE	3"	DBL 2X OR (1) 4X
SW2	(1) SIDE	2"	(1) 4X
SW4-DBL	BOTH SIDES	4"	DBL 2X OR (1) 4X

SHEAR WALL FRAMING SPECS:

1. PROVIDE 7/16" THICK APA STAMPED EXPOSURE 1 OSB SHEATHING OR BTR. PROVIDE STUDS @ 16" O.C. MAXIMUM.

2. FASTEN ALL WALL PANELS W/ 8D NAILING @ 12" O.C. EXCEPT ON PANEL EDGES. SEE ABOVE FOR PANELS EDGE FASTENING SPECS.

3. BLOCK ALL WALL PANEL (OSB) EDGES. OVERLAP PANELS OVER TOP AND BOTTOM PLATES.

4. FASTEN SILL PLATES TO WOOD FLOORS W/ 16D @ 4" O.C. STAGGERED.

5. FASTEN ALL DBL 2X STUD MEMBERS TOGETHER WITH 16D @ 6" O.C. STAGGERED.

6. TOP PLATES ARE CONTINUOUS WITH A 12" OVERLAP MIN. OR PROVIDE SIMPSON 16 GA. CS16 STRAPPING WITH A 12" MIN. LAP.

SHEAR WALL DIAGRAM

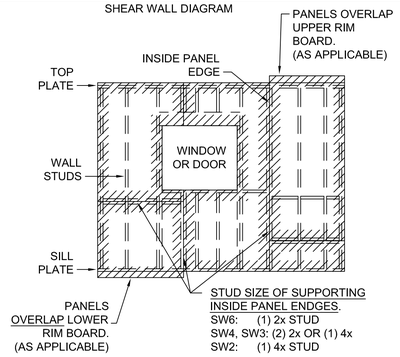


TABLE OF EQUIVALENT FASTENERS

NAIL SIZE	NAIL SPACING	STAPLE SPACING			NOTES
		14 GA.	15 GA.	16 GA.	
8D		14 GA.	15 GA.	16 GA.	ALL STAPLES ARE 2" LONG MINIMUM
	12"	12"	10"	8"	
	10"	10"	8"	6-1/2"	
	6"	6"	5"	4"	
	4"	4"	3-1/2"	2-1/2"	
	3"	3"	2-1/2"	1-1/2"	
10D		14 GA.	15 GA.	16 GA.	
	12"	9-1/2"	8"	6-1/2"	
	10"	8"	7"	5-1/2"	
	6"	5"	4"	3-1/2"	
	4"	3"	2-1/2"	2"	
	3"	2-1/2"	2"	1-1/2"	

FRAMING NAILING SCHEDULE U.N.O.

	CONNECTION DESCRIPTION	NAILING	NOTES
1	JOIST TO TOP PLATE OR BEAM	(2) 16D	TOENAIL
2	BLOCKING (BLKG) TO SILL OR BEAM	16D @ 4"O.C.	TOENAIL
3	SILL TO JOIST, RJ, BLKG, BEAM THRU FLOOR	16D @ 4" O.C.	FACE NAIL
4	TOP OR SILL PLATE TO STUD	(2) 16D	FACE NAIL
5	BUILT UP (MULTIPLE) STUDS / COLUMNS	16D @ 16" O,C.	FACE NAIL
6	DBL. TOP PLATE LAP	16D @ 4" O.C.	FACE NAIL
7	DBL. TOP PLATE INTERSECTION LAP	(2) 16D	FACE NAIL
8	HEADERS, BEAMS, GIRDERS MULTIPL/BUILT UP	(2) 16D @ 16"	FACE NAIL
9	OSB PANELS TO 2X WALL FRAMING	8D @ 6" / 12"	EDGE/INT.

ALL FASTENERS WHICH ARE TO BE INSTALLED IN PRESERVATIVE TREATED WOOD SHALL MEET THE REQUIREMENTS OF IBC 2304.10.

ROOF PANEL SCHEDULE

SEE NOTES 1,2,3

ROOF DESIGN SNOW LOAD	PANEL THICKNESS
UP TO 40 PSF	7/16"
UP TO 70 PSF	1/2"
UP TO 130 PSF	5/8"
FOR ROOF SNOW LOAD SEE DESIGN CRITERIA THIS SHEET.	

ROOF PANEL FASTENING TABLE

NAILING SPEC WHERE PANEL MEETS MEMBER, I.E. TRUSS, JOIST, BLOCK. (SEE DIAGRAM BELOW)	10D O.C. NAILING	
	PANEL EDGE	PANEL FIELD
6/12	6"	12"
6/6	6"	6"
3/3	3"	3"

NOTES (TYP.)

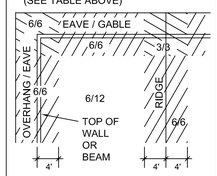
1. ALL ROOF PLANELS SHALL BE APA STAMPED, EXPOSURE 1, & RATED AS FOLLOWS:

THICKNESS SPAN RATING
7/16" 24/16
1/2" 32/16
5/8" 40/20

2. PROVIDE EDGE NAILING APPROX. 1/2" FROM PANEL EDGES W/ INSIDE EDGES SHARING INSIDE ROOF TRUSS, JOISTS & BEAMS. PROVIDE EDGE NAILING ALONG ALL BLOCKING.

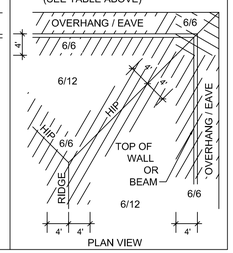
3. PROVIDE A MAX. JOIST OR TRUSS SPACING OF 24" O.C.

GABLE END ROOF NAILING SCHEDULE (SEE TABLE ABOVE)



PLAN VIEW

HIP ROOF NAILING SCHEDULE (SEE TABLE ABOVE)



PLAN VIEW

ROOF TRUSS & JOIST NOTES

TRUSSES (PRE-MANUFER'D)	1. ENGINEERING FOR TRUSSES IS PROVIDED BY THE MANUFACTURER. 3. PROVIDE METAL MANUFACTURED HURRICANE TIES AT ALL TRUSS BEARING LOCATIONS, H1 MIN, U.N.O. (SEE PLAN) 4. PROVIDE FULL HEIGHT BLOCKING AT TRUSS BEARING LOCATIONS EVERY OTHER TRUSS SPACING MINIMUM. 5. THESE PLANS HAVE BEEN ENGINEERED BASED ON THE ROOF FRAMING PLANS PROVIDED. TRUSS LAYOUTS DIFFERING FROM THE ROOF FRAMING PLANS SHOULD BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER. 6. THE TRUSS PACKAGE DESIGN SHALL BE SUBMITTED TO THE BUILDING OFFICIAL AS A DEFERRED SUBMITTAL PER IBC 2303.4. PRIOR TO PACKAGE SUBMITTAL, IT SHALL BE REVIEWED BY THE ENGINEER OF RECORD AND STAMPED FOR GENERAL CONFORMANCE.
ROOF JOISTS	1. PROVIDE SIMPSON VPA CONNECTORS AT ALL JOIST BEARING LOCATIONS. 2. PROVIDE FULL HIEGHT BLOCKING EVERY OTHER JOIST.

FLOOR SHEATHING NOTES

FLOOR SHEATHING U.N.O.:

1. PROVIDE 23/32" 'APA' STAMPED, EXPOSURE 1, T&G OSB PANELS FASTEN WITH 10D COMMON OR GALVANIZED NAILS @ 6"/ 12" EDGE / FIELD O.C.
2. PROVIDE CONTINUOUS GLUE JOIST TO PANELS.

WALL STUD HEIGHTS & LOAD BLOCKING

FRAMED WALL STUD HEIGHTS (HEIGHTS ARE UNSUPPORTED CLEAR SPANS) SEE NOTES 1,2

WALL STUDS MAX. HEIGHT	NOMINAL DF SIZE	VERT. O.C. SPACING	LATERAL SUPPORT
UP TO 10'	2 X 4	16"	NONE
UP TO 12'	2 X 4	12"	OSB, PLYWOOD, OR GLUED GYPSUM BOARD
UP TO 14'	2 X 6	16"	
UP TO 16'	2 X 6	12"	
UP TO 18'	2 X 6	8"	
UP TO 17'	1-1/2"x5-1/2"*	16"	
UP TO 18'	1-1/2"x5-1/2"*	12"	
UP TO 20'	1-1/2"x7-1/4"*	16"	

*TJI TIMBERSTRAND LSL STUDS. CONTACT ENGINEER FOR OTHER BRANDS OR CONFIGURATIONS.

NOTES (TYP.)

1. PLAN WALL SPECS, DIMENSIONS AND SCALES GOVERN WALL SIZING.

2. SEE MINIMUM NAILING SCHEDULE TABLE WHEN USING WALL STUD HT. TABLE.

3. PROVIDE 2X DF OR BTR. SQUASH BLOCKING UNDER ALL POINTS LOADS TRANSFERRED THROUGH WALLS FROM BEAMS AND GIRDER TRUSSES SUPPORTED BY MORE THEN (1) TRIMMER (SEE BEAM SCHEDULE) AND WHEN NOT LOCATED ALONG EXTERIOR WALLS WHEN RIM JOISTS ARE NOT PROVIDED.

DESIGN CRITERIA

GOVERNING CODE: 2018 IBC, ASCE 7
RISK CATEGORY II

1. LIVE LOADS:
SLEEPING AREAS 30 PSF
DECKS 60 PSF
BALCONIES 100 PSF
OTHER AREAS 40 PSF
ROOF 20 PSF

2. SNOW LOAD:
GROUND Pg 45 PSF
ROOF Ps 32 PSF + DRIFT
Pf: 32 PSF
Cs: 1.0 Ce: 1.0
Ct: 1.0 I: 1.0

3. WIND:
DESIGN SPEED:
120 MPH 3 SEC. GUST
EXPOSURE: C

4. SEISMIC:
SDC: D R: 1.5 I: 1.0
SITE CLASS: D (ASSUMED)
Sms: 0.98
Sm1: 0.52
Sds: 0.65

FORCE RESISTING SYSTEM:
WOOD LIGHT FRAME W/ WOOD PANEL

ANALYSIS PROCEDURE:
SIMPLIFIED (IBC 12.14.8)

5. SOIL BEARING: ALLOWABLE PRESSURE ON NATIVE SOIL:
1500 PSF (ASSUMED)

6. DEAD LOADS:
ROOF 15 PSF (ASPHALT SHINGLES)
FLOOR 15 PSF (INCL. THIN TILE)
WALL 10 PSF (STUCCO, SIDING)
WALL 45 PSF (VENEER: BRICK,STONE)
CONCRETE 145 PCF (FTGS, FOUND, ETC)

BEAM SCHEDULE EX

SEE NOTES 1,2,3,4,5

DESIGNATION (SEE PLAN)	BEAM DESCRIPTION	END QTY.	TRIMMER U.N.O.	TYPE U.N.O.
MIN. BEAM OR HEADER U.N.O.	(2) 2 X 10 DF#2		1	2X4
B(2)5.1/2	(2) 1-3/4" X 5-1/2" LVL		1	2X4
B(2)6	(2) 2 X 6 DF/HF#2		1	2X4
B(2)7.1/4	(2) 1-3/4" X 7-1/4" LVL		1	2X4
B(3)7.1/4	(3) 1-3/4" X 7-1/4" LVL		1	2X4
B(2)8	(2) 2 X 8 DF/HF#2		1	2X4
B(3)8	(3) 2 X 8 DF/HF#2		1	2X6
B(2)9.1/2	(2) 1-3/4" X 9-1/2" LVL		3	2X4
B(2)9.1/2	(2) 1-3/4" X 9-1/2" LVL		2	2X6
B(3)9.1/2	(3) 1-3/4" X 9-1/2" LVL		3	2X6
B(2)10	(2) 2X10" DF/HF #2		1	2X4
B(3)10	(3) 2 X 10 DF/HF#2		1	2X4
B(1)11.7/8	(1) 1-3/4" X 11-7/8" LVL		2	2X4
B(2)11.7/8	(2) 1-3/4" X 11-7/8" LVL		3	2X4
B(3)11.7/8	(3) 1-3/4" X 11-7/8" LVL		4	2X6
B(4)11.7/8	(4) 1-3/4" X 11-7/8" LVL		5	2X6
B(2)12	(2) 2 X 12 DF/HF#2		1	2X4
B(2)14	(2) 1-3/4" X 14" LVL		4	2X4
B(3)14	(3) 1-3/4" X 14" LVL		4	2X6
B(2)16	(2) 1-3/4" X 16" LVL		4	2X4
B(3)16	(3) 1-3/4" X 16" LVL		5	2X6
B(2)18	(2) 1-3/4" X 18" LVL		5	2X4
B(3)18	(3) 1-3/4" X 18" LVL		5	2X6
GLB	WOOD GLULAM BEAM.	SEE PLAN		
W#X##	STEEL WIDE FLANGE BEAM	SEE PLAN		

1. JOIN MULTIPLE PIECE BEAMS W/ (2) 16D @ 12" O.C.
2. PROVIDE METAL FRAMING CONNECTORS (I.E. SIMPSON #CB) TO TIE BEAMS GREATER THAN 6'-0" TO POSTS OR TRIMMERS.
3. LVL IS LAMINATED VENEER LUMBER, I.E., MICROLAM, VERSALAM. MINIMUM LVL SPEC PROVIDE 1.9E AND 2600 PSI Fb OR BTR.
4. GLB IS GLULAM BEAM. PROVIDE 24F-V4 SPEC U.N.O.
5. ALL STEEL BEAMS TO MEET ASTM A992 OR Fy = 50 ksi, Fu = 65 ksi CRITERIA.
6. X-BEAM IS ROSBORO X-BEAM BRAND GLULAM BEAMS OR EQUIV.

7. ALL BEAMS AND HEADERS MAY HAVE UP TO A 1-1/2" DIAMETER HOLE THRU THE ANY LOCATION OF THE SPAN, BUT NOT WITHIN 1" OF THE TOP OF BOTTOM OF THE DEPTH.

POST & COLUMN SCHEDULE

DESIGNATION (SEE PLAN)	DRESSED SECTION SIZE (APPROX) & GRADE (1)	CAP/BASE U.N.O.(2)	BASE TO CONC. U.N.O.(2)
P4X4	3-1/2" X 3-1/2" DF#1	BC4, LCE4	ABU44Z
P4X6	3-1/2" X 5-1/2" DF#1	BC46, LCE4	ABU46Z
P6X6	5-1/2" X 5-1/2" DF#1	BC6, AC6	ABU66Z
P6X8	5-1/2" X 7-1/2" DF#1	BC_, AC6	SEE PLAN
P8X8	7-1/2" X 7-1/2" DF#1	BC_, AC6	ABU88Z
P10X10	9-1/2" X 9-1/2" DF#1	SEE PLAN	ABU1010Z
P12X12	11-1/2" X 11-1/2" DF#1	SEE PLAN	ABU1212Z
P6X6RS	6" X 6" DF#1	AC6	ABU66RZ
P8X8RS	8" X 8" DF#1	SEE PLAN	ABU88RZ
P10X10RS	10" X 10" DF#1	SEE PLAN	ABU1010RZ
P12X12RS	12" X 12" DF#1	SEE PLAN	ABU1212RZ

1. DF#1 = DOUG FIR GRADE #1 OR BETTER
2. CONNECTOR MODEL NUMBERS RELATE TO SIMPSON STRONG TIE OR USP (MITEK) BRAND WHERE APPLICABLE. OTHER CONNECTOR MODEL NUMBERS ACCEPTABLE IF ALL RATED CAPACITIES EXCEED THOSE LISTED. (LCE_) & (AC_) MODELS ARE INSTALLED IN PAIRS.
U.N.O. = UNLESS NOTED OTHERWISE (ON THE PLANS).

NOTE: IF ENGINEER'S STAMP AND SIGNATURE ARE NOT SHOWN IN THIS BOX, THIS PLAN SHALL BE CONSIDERED A DRAFT AND NOT FOR CONSTRUCTION

PRICE ENGINEERING INC.
STRUCTURAL • CIVIL
3677 NORTH HWY 126 SUITE 4B FARR WEST, UT 84404

PROJECT TITLE:
Richmond City North Pavilion
Approx. 200 W Main Street
TAX ID 09-058-0016
Richmond, Utah

MARK	DESCRIPTION	DATE
	DESIGN SET	02/20/2023

03/28/2023

DESIGNED BY:
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KYLE@PRICE-ENGINEERING.COM

SHEET TITLE:
STRUCTURAL SCHEDULE AND NOTES

SD3

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